

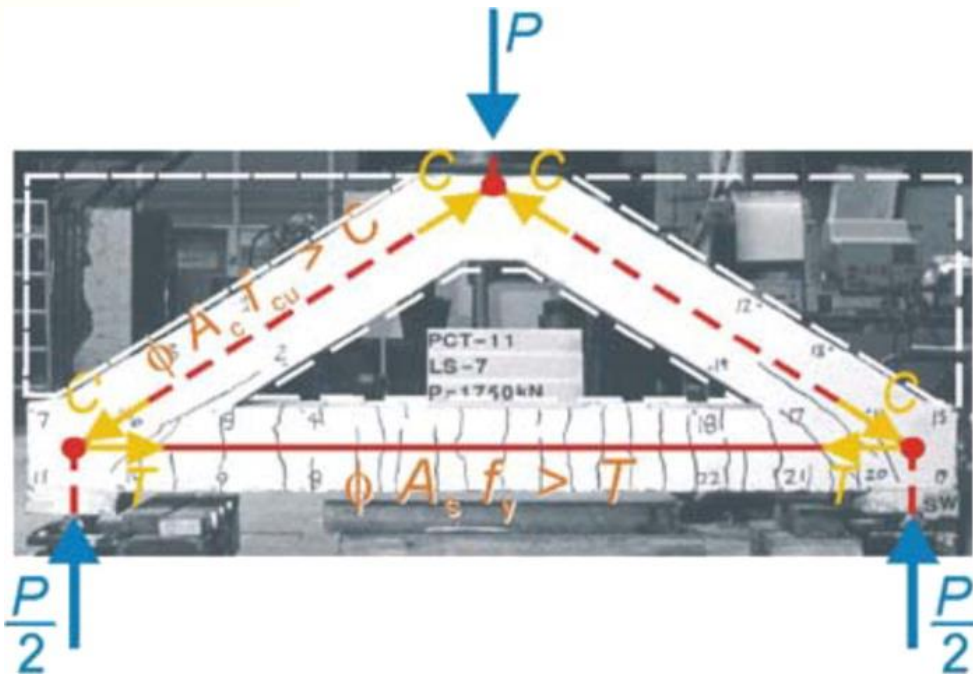
5



Advanced RC Structures

PART 3

Strut-and-Tie Model



Deep Beam

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SURANAREE

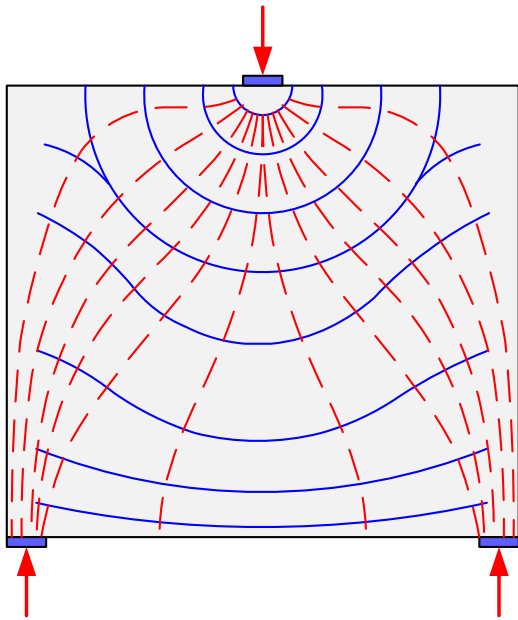
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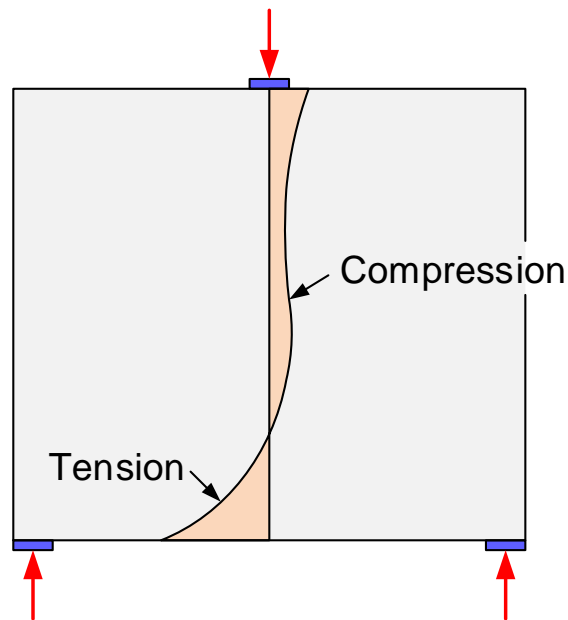
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Deep Beam

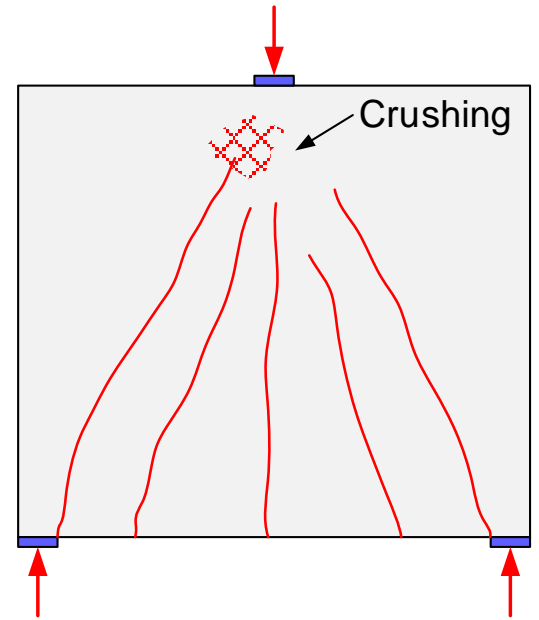
Deep beam with one point load on top



Stress trajectories

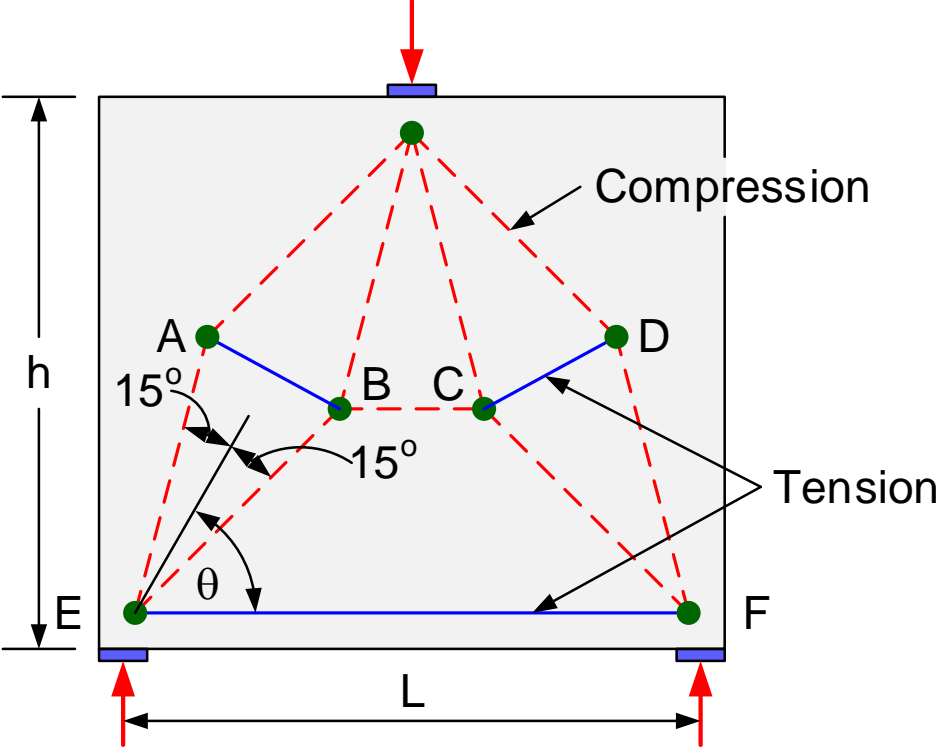


Stress distribution
@ mid-span section

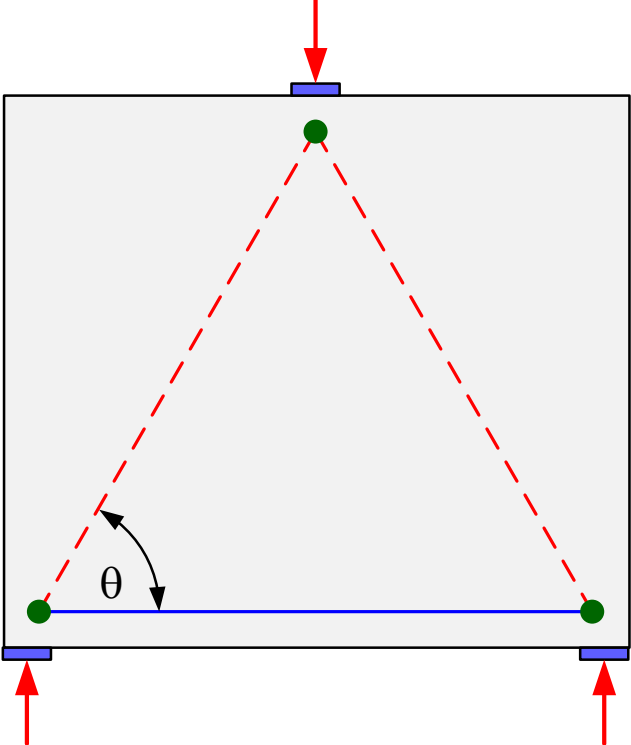


Crack pattern

Deep Beam

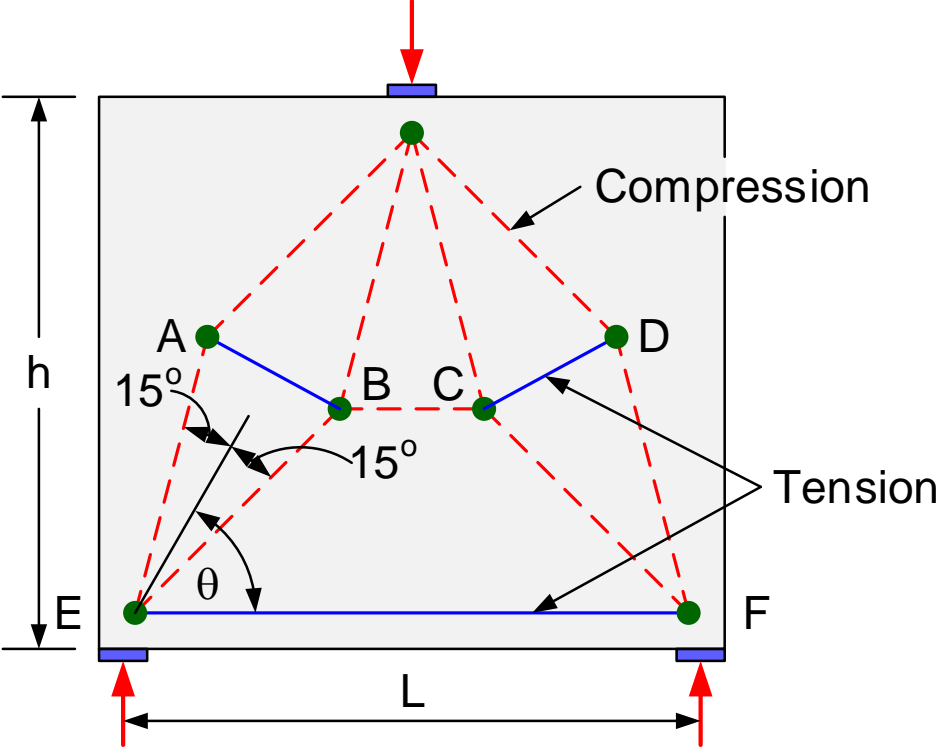


Truss Model

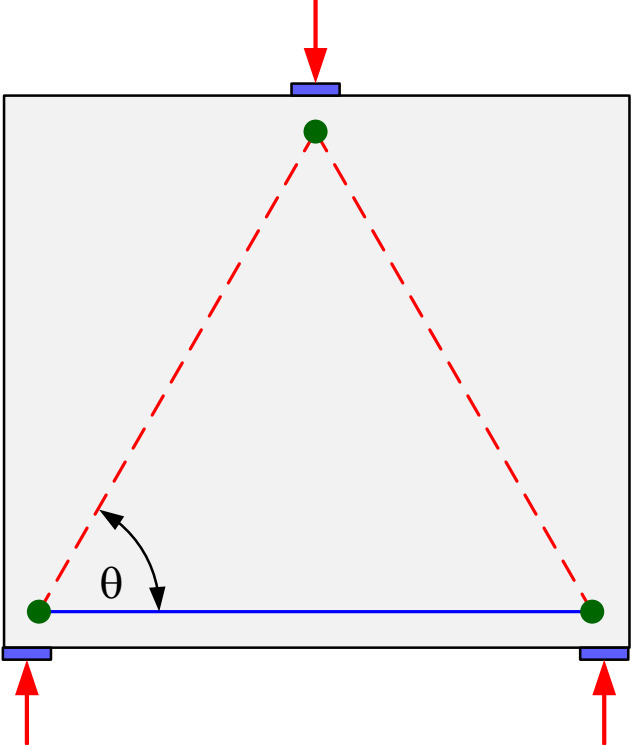


Simple Truss Model

Deep Beam

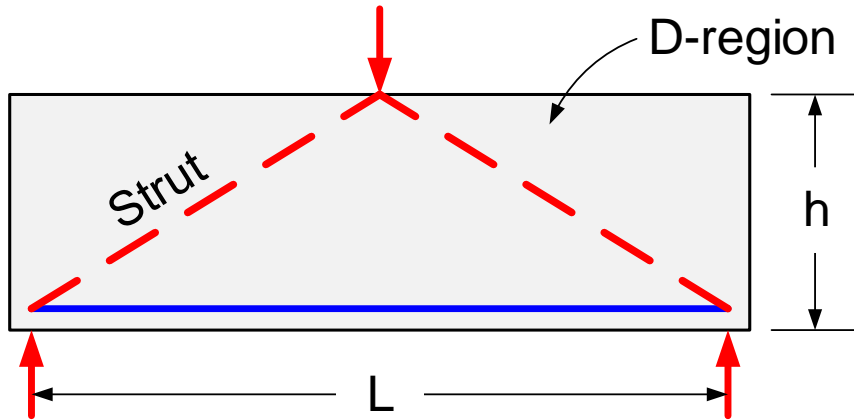


Truss Model



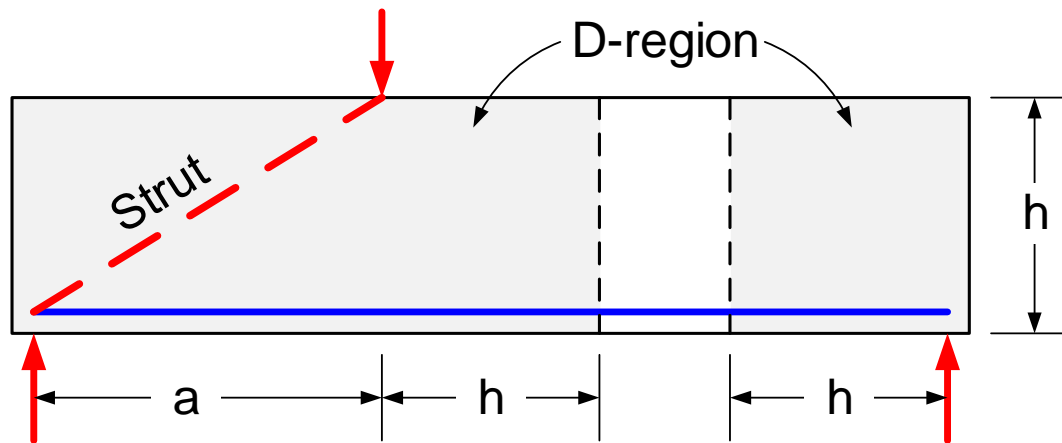
Simple Truss Model

Deep Beam



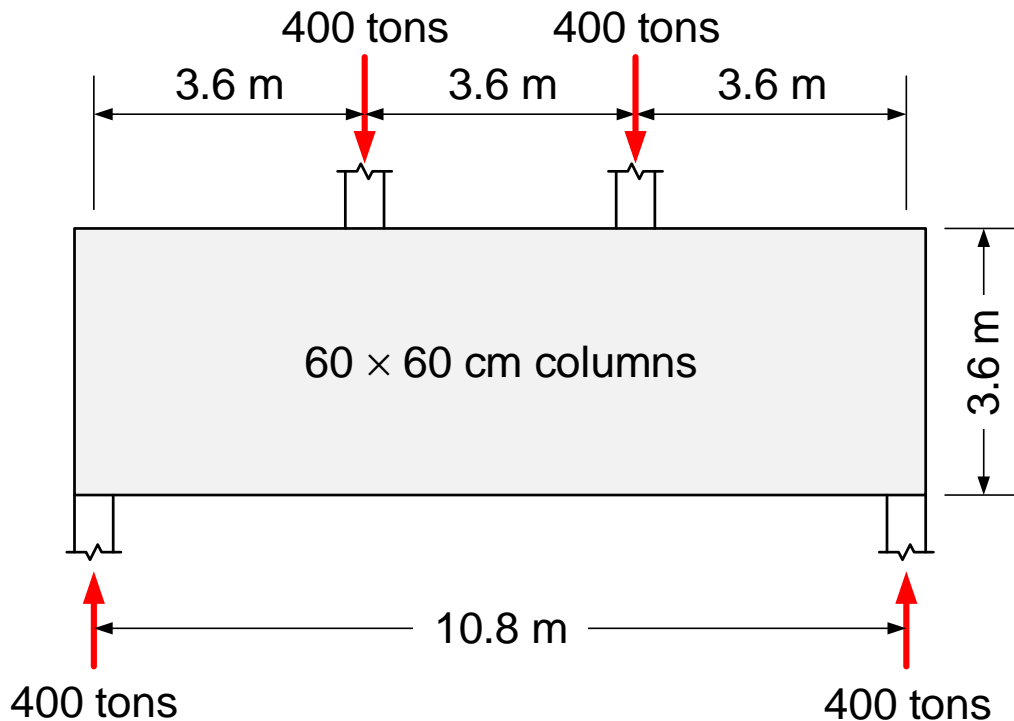
Deep beam with $L \leq 4h$

Beams with clear spans less than or equal to 4 times the total member depth or with concentrated loads placed within twice the member depth of a support are classified as deep beams



Deep beam with $L > 4h$

EX1 : Deep Beam A transfer girder is to carry two 60 cm. square columns, each with factored loads of 400 tons located at the third points of its 10.8 m span. The beam has a thickness of 0.6 m and a total height of 3.6 m Design the beam for the given loads, ignoring the self-weight, using $f'_c = 280$ ksc and $f_y = 4,000$ ksc.



วิธีทำ (1) Span-to-depth ratio:

$$L/h = 10.8/3.6 = 3 < 4$$

∴ Can be considered as deep beam

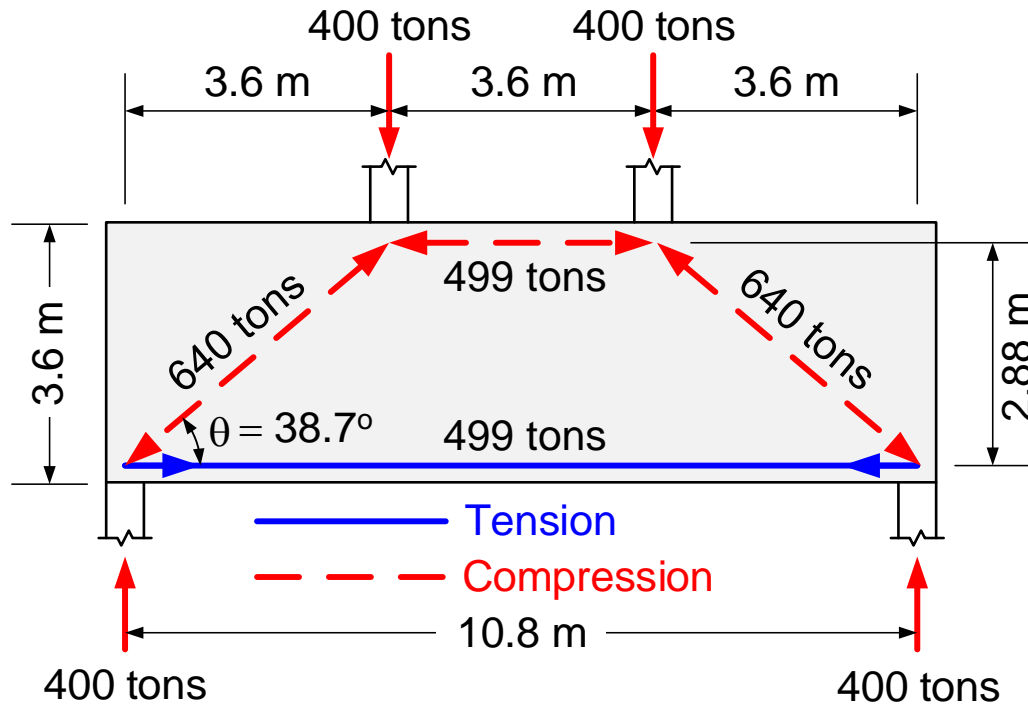
$$\text{Assume } d = 0.9h = 3.24 \text{ m}$$

(2) Max shear capacity:

$$\begin{aligned} V_n &= 2.65\sqrt{280} \times 60 \times 324 / 10^3 \\ &= 862 \text{ tons} \end{aligned}$$

$$V_u/\phi = 400/0.75 = 533 < V_n \text{ OK}$$

(3) Truss Model:



Assume distance between top strut & ties:

$$0.8 h = 0.8(3.6) = 2.88 \text{ m}$$

$$\theta = \tan^{-1}(2.88 / 3.6) = 38.7^\circ$$

Diagonal strut compression:

$$F_{uc} = 400 / \sin 38.7^\circ = 640 \text{ tons}$$

Top strut & tie tension:

$$F_{ut} = 400 / \tan 38.7^\circ = 499 \text{ tons}$$

(4) Size of Struts & Nodal zones:

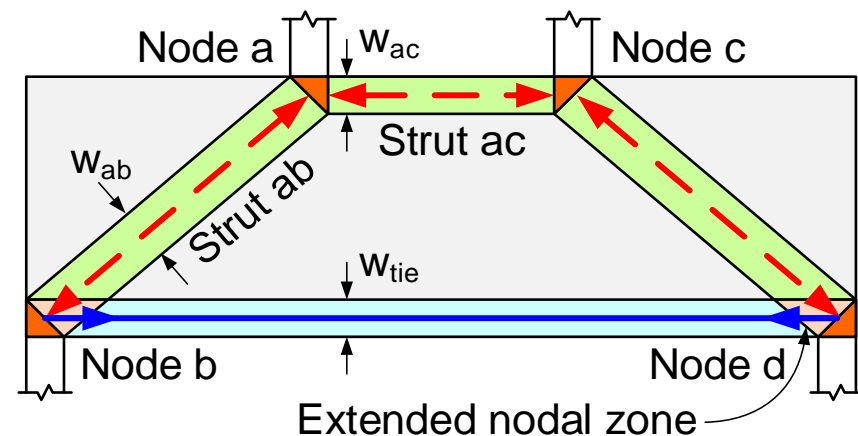
@ Node a & c, nodal stress under column

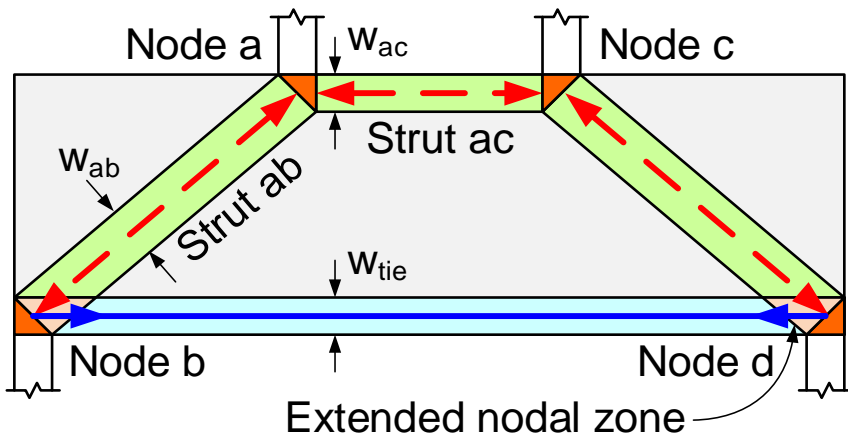
$$f_{un} = 400 \times 10^3 / (60 \times 60) = 111 \text{ ksc}$$

For node C-C-C, $\beta_n = 1.0$

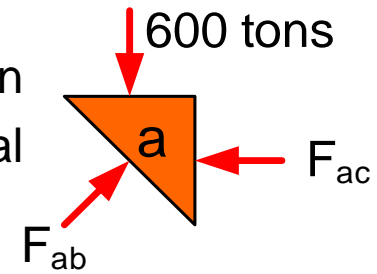
$$\phi f_{ce} = \phi 0.85 \beta_n f'_c = 0.75 \times 0.85 \times 1.0 \times 280$$

$$= 179 \text{ ksc} > f_{un} \text{ OK}$$





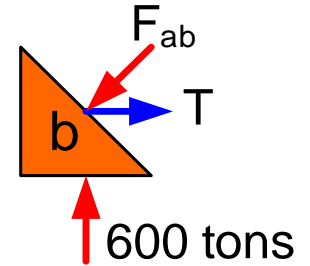
For hydrostatic compression nodal, areas are proportional to compressive forces.



$$w_{ac} = F_{ac}/(b_s \times f_{un}) = 499 \times 10^3 / (60 \times 111) = 75 \text{ cm}$$

$$w_{ab} = F_{ab}/(b_s \times f_{un}) = 640 \times 10^3 / (60 \times 111) = 96 \text{ cm}$$

$$w_{tie} = T/(b_s \times f_{un}) = 499 \times 10^3 / (60 \times 111) = 75 \text{ cm}$$



Center-to-center distance between top strut & tie = $3.6 - 0.75 = 2.85 \text{ m} \approx 2.88 \text{ m}$ **OK?**

(5) Strength of Struts: $\phi F_{ns} = \phi 0.85 \beta_s f'_c w_i b_s$

Assume uniform section for top strut and bottle-shaped for diagonal struts.

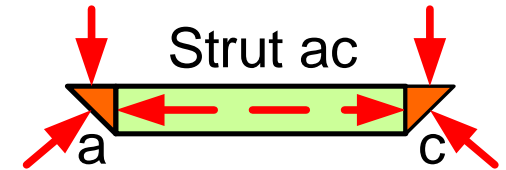
$$\begin{aligned} \text{Strut ac } (\beta_s = 1.0): \quad \phi F_{ns} &= 0.75 \times 0.85 \times 1.0 \times 280 \times 75 \times 60 / 1,000 \\ &= 803 \text{ tons} > 499 \text{ tons} \quad \mathbf{OK} \end{aligned}$$

$$\begin{aligned} \text{Strut ab } (\beta_s = 0.75): \quad \phi F_{ns} &= 0.75 \times 0.85 \times 0.75 \times 280 \times 96 \times 60 / 1,000 \\ &= 771 \text{ tons} > 640 \text{ tons} \quad \mathbf{OK} \end{aligned}$$

(6) Strength of Nodal Zones: $\phi F_{nn} = \phi 0.85 \beta_n f'_c w_i b_s$

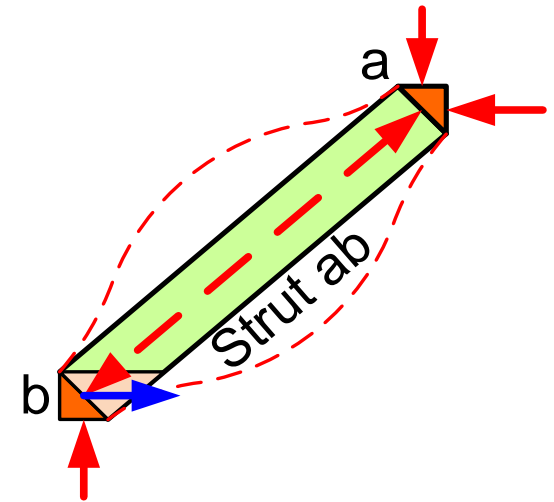
At ends of strut ac, node a & c (C-C-C) $\beta_n = 1.0$,

$$\begin{aligned} \phi F_{nn} &= 0.75 \times 0.85 \times 1.0 \times 280 \times 75 \times 60 / 1,000 \\ &= 803 \text{ tons} > 499 \text{ tons} \quad \text{OK} \end{aligned}$$

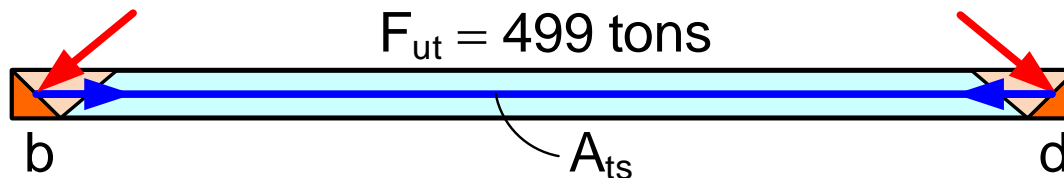


At ends of strut ab, node a (C-C-C) $\beta_n = 1.0$, node b (C-C-T) $\beta_n = 0.8$, **control**

$$\begin{aligned} \phi F_{nn} &= 0.75 \times 0.85 \times 0.8 \times 280 \times 96 \times 60 / 1,000 \\ &= 823 \text{ tons} > 640 \text{ tons} \quad \text{OK} \end{aligned}$$

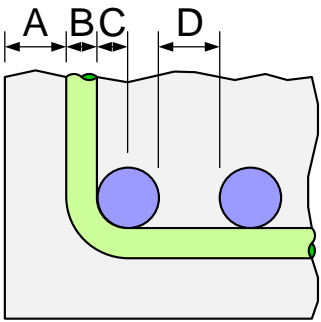


(7) Design of Ties and Anchorage:



Steel area of tie: $A_{ts} = F_{ut} / \phi f_y = 499 / (0.75 \times 4.0) = 166 \text{ cm}^2$

USE 17-DB36 ($A_{ts} = 173 \text{ cm}^2$)



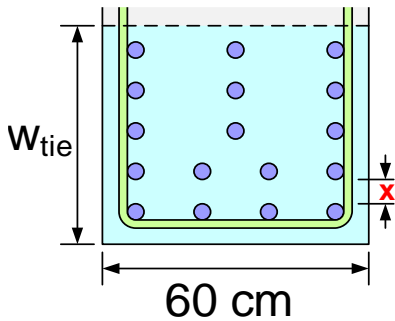
Steel detailing for DB36:

$A = 4 \text{ cm}$, $B = 1.2 \text{ cm}$, $C = 1.8 \text{ cm}$ and $D = 3.6 \text{ cm}$

Max. number of DB36 in one layer:

7-DB36: $b = 7(3.6) + 2(4+1.2) + 6(3.6) = 57.2 \text{ cm} < 60 \text{ cm}$ **OK**

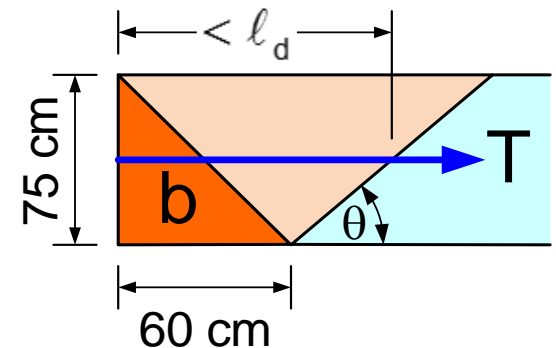
USE 5 layers of steel $4+4+3+3+3 = 17\text{-DB36}$ (trial&error)



Tie Width: $w_{\text{tie}} = 5(3.6) + 4(\mathbf{11.6}) + 2(4+1.2)$
 $= 74.8 \text{ cm} \approx 75 \text{ cm}$ **OK**

Anchorage length of DB36: $l_d = \frac{0.19f_y}{\sqrt{f'_c}} d_b$
 $= 45.4 d_b = 163.5 \text{ cm}$

Nodal + Extended zones:
 $= 60 + 0.5 \times 75 \cot 38^\circ$
 $= 108 \text{ cm} < l_d$



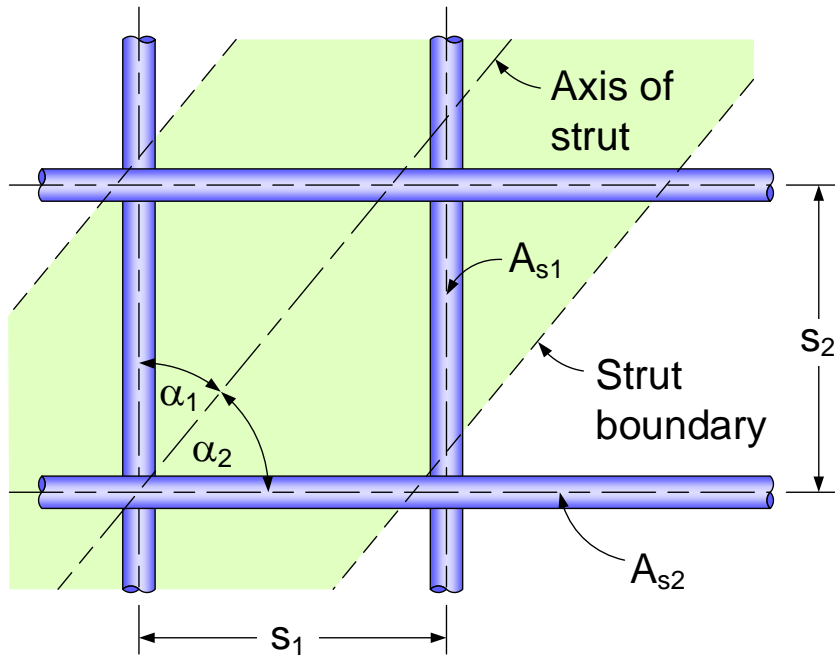
\therefore Need 90° hook or mechanical anchor

(8) Shear Reinforcements: using DB12: $A_v = A_h = 2.26 \text{ cm}^2$ and $s_1 = s_2$

$$\frac{A_v}{s_1} = \frac{A_v}{s_2} = 0.0025b_w = 0.0025(60) = 0.15 \text{ cm}^2/\text{cm}$$

$$s_1 = s_2 = 2.26/0.15 = 15.1 \text{ cm} \quad \text{USE DB12 @ 0.15 m \# E.F.}$$

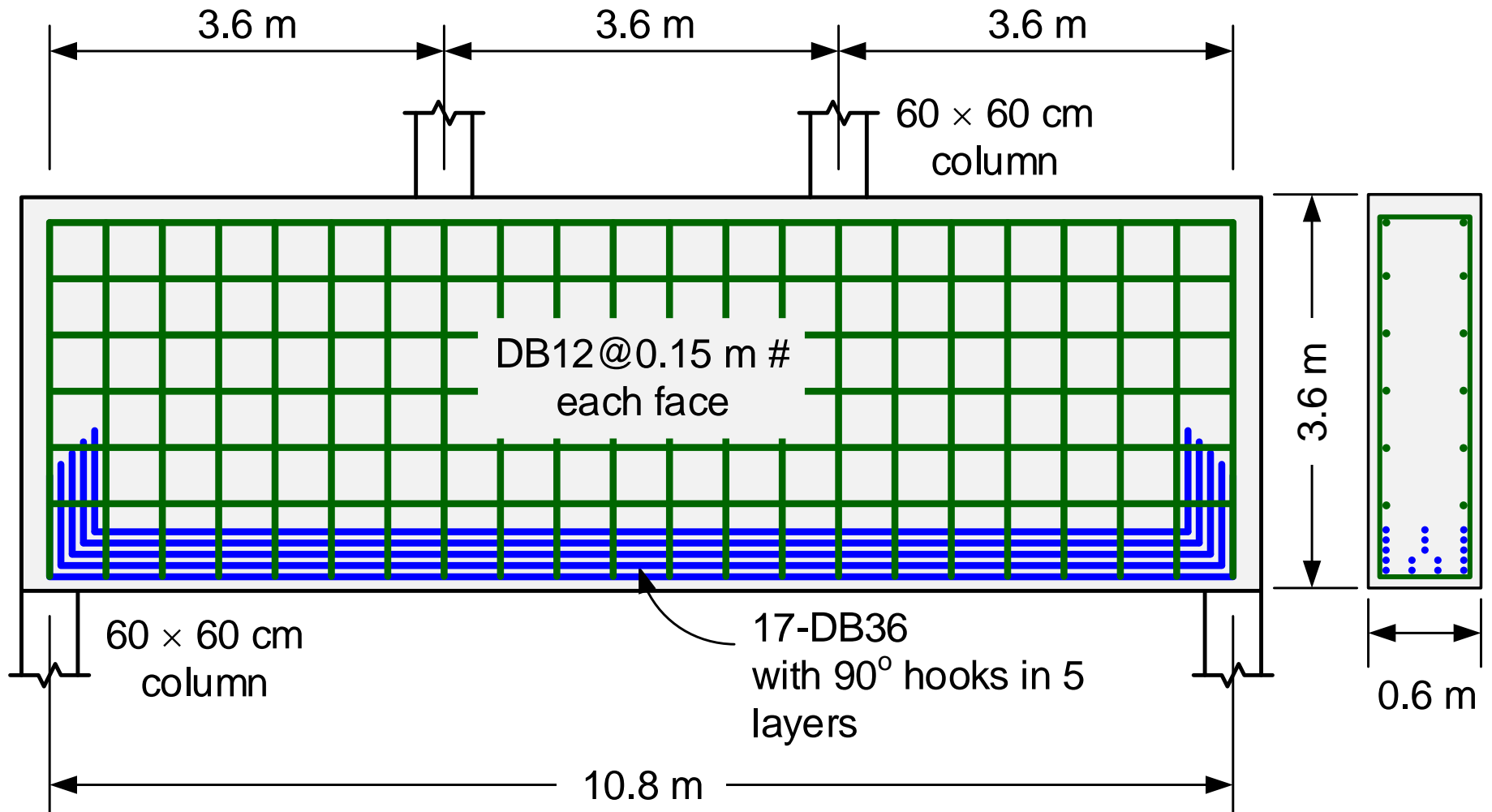
Check splitting of bottle shape strut: $\sum \frac{A_{si}}{bs_i} \sin \alpha_i \geq 0.003$



$$\frac{2.26 \sin 38^\circ + 2.26 \sin 52^\circ}{60 \times 15}$$

$$= 0.0035 > 0.003 \quad \text{OK}$$

(9) Reinforcement details:



End of Lecture